M.G.Kalvanshetti, G.S. Mirajkar / International Journal of Engineering Research and Applications (IJERA) ISSN: 2248-9622 www.ijera.com Vol. 2, Issue 6, November- December 2012, pp.1460-1464 **Comparison Between Conventional Steel Structures And Tubular Steel Structures**

M.G.Kalvanshetti, G.S. Miraikar**

*(Assistant Professor, Walchand Institute of Technology, Solapur, India) **(P.G Student-, Civil- Structurs civil Engineering Department, Walchand Institute of Technology, Solapur, India)

ABSTRACT

Most of the steel structures are builtedup with conventional sections of steels which are designed and constructed by conventional methods. This leads to heavy or uneconomical structures. Tubular steel sections are the best replacements to the conventional ones with their useful and comparatively better properties. It is obvious that due to the profile of the tube section, dead weight is likely to be reduced for many structural members .which derives overall economy. This study is regarding the economy, load carrying capacity of all structural members and their corresponding safety measures. Economy is the main objective of this study involving comparison of conventional sectioned structures with tubular sectioned structure for given requirements. For study purpose superstructure-part of an industrial building is considered and comparison is made. Study reveals that, up to 40 to 50% saving in cost is achieved by using tubular sections.

Key words -, IS 800, IS 806, IS 875, Tubular sections.

INTRODUCTION 1.0

This study is about designing components of industrial buildings using conventional steel sections, square tube, circular tube and rectangular Tube sections and assigns most suitable section according to its dead weight and ease of connection methods. Experimental and theoretical studies have been carried by, S. Poonaya, U.Teeboonma, C.Thinvongpituk [9] regarding plastic collapse analysis of thin walled circular tubes subjected to bending. Design of circular steel arch with hollow sections is presented by C.A. Demopoulos, C.J. Gantes [10], they have suggested that Tubular sections are an economical, efficient and strong alternative to conventional sections used in steel structure.

1.1 **ADVANTAGES** OF **TUBULAR SECTIONS:**

- 1. For tubular sections, higher strength to weight ratio could result in upto 30% savings in steel.
- 2. Due to the high torsional rigidity and compressive strength, Tubular sections behave more efficiently than conventional steel section.

- 3. For dynamic loads tubes have higher frequency
- of vibration than any other rolled section.
- 4. Ease of maintenance.
- 5. Free from sharp edges.
- 6. Ease of fabrication and erection.

2.0 **PROBLEM:**

To determine the effectiveness of tubular sections an industrial shed is considered Analysis and design is carried out using conventional steel sections and tubular steel sections. In tubular circular, rectangular and square shapes are considered. Cost comparison is made for all above sections.

Following data is considered for analysis and design of industrial shed.

Data for Industrial building

1) Plan area around:- 800 sq.m.

Location: -Solapur, MIDC Area, 2) Maharashtra --India.

- Roof truss:-Modified Howe type 3)
- 4) Geometry of truss:- span 24 m, θ =16.26°, 5) 10 panel Points spacing of purlins 1.425 m.
- 6) Length of sheet=3.05m, sloping length=12.5 m.
- 7) Spacing of truss = 5m, No of trusses = 8Configuration of truss is shown in fig 1.



Figure1: Geometry of truss

3.0 APPROACH:

- 1. Dead load analysis is done according to IS 875 (Part1) with the help of STAAD-PRO
- 2. Live load analysis is done according to IS 875 (Part2) with the help of STAAD-PRO

M.G.Kalyanshetti, G.S. Mirajkar / International Journal of Engineering Research and Applications (IJERA) ISSN: 2248-9622 www.ijera.com Vol. 2, Issue 6, November- December 2012, pp.1460-1464

- 3. Wind load analysis is done according to IS 875 (part 3) with the help of STAAD-PRO
- 4. Designing is done according to IS 800,IS806 and STAAD PRO

Conventional design is carried out as per IS 800 and tubular sectioned design is carried out as per IS 806.

4.0 RESULTS AND DESCUSSIONS

- A) Total Dead Load.
- 1) On central purlin =3.133 kN
- 2) On intermediate purlin=2.8318 kN
- 3) On end purlin=1.5133 kN
- B) Total Live Load
- 1) On central purlin =2.849 kN
- 2) On intermediate purlin=2.5241 kN
- 3) On end purlin $[1.1]/2 \times \cos 16.26 \times 5 \times 0.41653 =$
- 1.099kN C) Total Wind Load
- 1) On central purlin= -7.5981 kN
- 2) On intermediate purlin=-6.533 kN
- 3) On end purlin=-2.846 kN

Using above results design is carried out for required load carrying capacity. Optimum sections are assigned to truss members and purlin members.Comparison is made for self weight and cost of various elements of truss such as principal rafter,tie member, strut member, sling member, purlin member. Results for single truss are presented graphically in graph 1 to graph 5. These results shows that considerable amount of saving is achieved using Tubular sections

Also analysis is carried out for total shed area of 800 sq.m. consisting 8 numbers of trusses. In this case also comparison is made for various elements of truss. Results are presented in tabular form i.e. table 1 to table 5. Study reveals that considerable saving in cost can be achieved by using tubular sections.

Graph 1. Variation of design weights for principal rafter of Modified Howe Truss







Graph 3. variation of design weights for sling member

M.G.Kalyanshetti, G.S. Mirajkar / International Journal of Engineering Research and Applications (IJERA) ISSN: 2248-9622 www.ijera.com Vol. 2, Issue 6, November- December 2012, pp.1460-1464 Design Weight of sling member for different sections Table 1: comparison for pr



Graph 4. variation of design weights for Strut Member



Graph 5. variation of design weights for Purlin Member



1	Table 1: comparison for principal ratter						
	section	conventi onal	Squar e Tube	Rect.t ube	Ci	rc. tube	
	Section	2ISA65x 65x6	80x80x 4.8	122x61 x4.5	0. N. ,t=	D.114.3.7, B.100 3.6	
	Weight in kg/m	2x14.9=2 9.8	9.66	11.88	6.4	12	
	Wight for 8 trusses(k g)	3814.4	1236.4	1520.6 4	82	822.08	
	Reductio n in kg		142.96 32	-54.83	119	98.75	
	Rate for 8 trusses Rs	113587.2	117084	127961	10	5019.2	
	Total saving Rs		-3497	- 143748	85	68	
	% save saving		3.07% (loss)	12.65 (loss)	7.5	54 saving	
	Table2: co	mparison fo	or strut me	mber		1.1	
	section	conventi onal	Square Tube	Rect.tub	be	Circ. tube	
	castion				-	-	
	section	ISA65x6 5x8	60x60x 3.2	80x40x 2	3.	O.D60.3, N.B50 ,t=3.6	
	Weight in kg/m	ISA65x6 5x8 7.7	60x60x 3.2 5.5	80x40x 2 5.5	3.	O.D60.3, N.B50 ,t=3.6	
	Weight in kg/m Wight for 8 trusses (kg)	ISA65x6 5x8 7.7 1624.51	60x60x 3.2 5.5 1160.48	80x40x 2 5.5 1160.48	3.	0.D60.3, N.B50 ,t=3.6 5.03 1061.20	
	Weight in kg/m Wight for 8 trusses (kg) Reductio n in kg	ISA65x6 5x8 7.7 1624.51	60x60x 3.2 5.5 1160.48 464.03	80x40x 2 5.5 1160.48 464.03	3.	0.D60.3, N.B50 ,t=3.6 5.03 1061.20 563.31	
	Weight in kg/m Wight for 8 trusses (kg) Reductio n in kg Rate for 8 trusses Rs	ISA65x6 5x8 7.7 1624.51 81225.76	60x60x 3.2 5.5 1160.48 464.03 63826.4	80x40x 2 5.5 1160.48 464.03 63826.4	3.	0.D60.3, N.B50 ,t=3.6 5.03 1061.20 563.31 58366.4	
	Section Weight in kg/m Wight for 8 trusses (kg) Reductio n in kg Rate for 8 trusses Rs Total saving Rs	ISA65x6 5x8 7.7 1624.51 81225.76	60x60x 3.2 5.5 1160.48 464.03 63826.4 17399.3 6	80x40x 2 5.5 1160.48 464.03 63826.4 17399.3	3.	O.D60.3, N.B50 ,t=3.6 5.03 1061.20 563.31 58366.4 22859.36	

M.G.Kalyanshetti, G.S. Mirajkar / International Journal of Engineering Research and Applications (IJERA) ISSN: 2248-9622 www.ijera.com Vol. 2, Issue 6, November- December 2012, pp.1460-1464

Fable 3:	comparison	for	Tie	member
----------	------------	-----	-----	--------

section	conventio nal	Square Tube	Rect.tu be	Circ. tube
	2ISA70x7 0x8	80x80x 4.8	122x61 x4.5	O.D.11 4.3,N.B. 100 ,t=3.6
Weight in kg/m	16.6	10.87	11.88	9.75
Wight for 8 trusses(kg)	3320	2174	2376	1950
Reductio n in kg		1146	944	1370
Rate for 8 trusses Rs	166000	119570	130680	107250
Total saving Rs		46430	35320	58750
% total saving		27.96%	21.27%	35.39%

n fon aling

Table 4. comparison for sing member					
section	convent ional	Square Tube	Rect.t ube	Circ. tube	
section	ISA50x 50x6	50x50x3.2	60x40 x2.9	O.D4 8.3,N. B40 ,t=4	
Weight in kg/m	4.5	4.12	4.12	4.37	
Wight for 8 trusses(kg)	1073.52	982.8	982.8	1042	
Reduction in kg		90.72	90.72	31.02	
Rate for 8 trusses (Rs)	53676	54054	54054	57337	
Total saving Rs		-378(loss)	-378 L	- 3661 L	
% total saving		0.7%(loss)	0.7% (loss)	6.8% (loss)	

 Table 5: comparison for Purlin member

section	conventi onal	Square Tube	Rect.tube	Circ. tube
	ISA125x 95x12	113.5x113.5 x4.8	122x61x 5.4	O.D.139 .7,N.B.1 25 ,t=4.5
Weight in kg/m	19.6	15.92	14.01	15.00
Wight for 20 purlins(kg)	13720	11144	9807	10500
Reducti on in kg		2576	3913	3220
Rate for 20 purlins Rs	686000	612920	539380	577500
Total savings		73080	146620	438500
% total saving		10.65%	21.37%	15.81%

4.1 TOTAL COST:

• Total cost for modified Howe truss using conventional sections: Rs 1,46496.62 /-. Total cost for modified Howe truss using square

tube sesections: Rs 74,962.83 /-. (% saving in cost =48.83%)

. Total cost for modified Howe truss using rectangular tubee sections: Rs 74034.26 /- (/% saving in cost=49.46%)

. Total cost for modified Howe truss using circuar tube sections: Rs 60996.68 /- (% saving=58.36%)

. 5.0 CONCLUSION

Above study reveals that tubular sections proves to be economical. Total saving of almost 50 % to 60 % in cost is achieved. Out of circular, square and rectangular shapes, due to connection difficulties of circular tube sections, it is suggested to adopt rectangular or square tube sections. This study is for a given area of 800 sq.m. and truss of modified howe type. Effectiveness of Tubular section can be verified for different plan areas for various types of trusses.

From above observations and results one can conclude that, the structural members having larger unsupported lengths can be assigned tubular sections which will derive overall economy. For smaller unsupported lengths one will have to assign minimum sections for both conventional and tubular sections so that economy is not considerably achieved. In such cases due to larger initial cost it becomes uneconomical. Initial cost for tubular sections is more however ,due to

M.G.Kalyanshetti, G.S. Mirajkar / International Journal of Engineering Research and Applications (IJERA) ISSN: 2248-9622 www.ijera.com Vol. 2, Issue 6, November- December 2012, pp.1460-1464

reduction in total dead weight, it is overall economical not only for industrial buildings but also for various steel structures like transmission towers, bridge structures etc.

REFERANCES

- 1. IS 800-1984, Code of practice for general construction in steel.
- IS 875-1987(part-1) Code of practice for design loads (other than earthquake) for buildings and structures.
- 3. IS 875-1987(part-2) Code of practice for design loads (other than earthquake) for buildings and structures?
- 4. IS 875-1987(part-3) code of practice for design loads (other than earthquake) for buildings and structures.
- 5. IS 806-code of practice for use of steel tubes) for buildings and structures.
- 6. Design of steel structure by S.K. DUGGAL.
- 7. Design of steel structure by Ashok Kumar Jain.
- 8. Broucher of Tata steel structures.
- "Plastic collapse analysis of thin walled circular tubes subjected to bending" by S. Poonaya, U.Teeboonma, C.Thinvongpituk, 15th may 2008.
- "Design of circular steel arch with hollow sections" by C.A. Demopoulos, C.J. Gantes_Department of Civil Engineering, National Technical University of Athens, Greece, 10 June 2007. cat